



# **GEOTEK CONSORTIUM**

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**GEOTECHNICAL INVESTIGATION REPORT  
PROPOSED DEVELOPMENT  
PALGHAR, BHOISAR (E)  
FOR M/S. U.S. HOLIDAY REALTY PVT. LTD.**

**Submitted to,  
M/S. U.S. HOLIDAY REALTY PVT. LTD.**

**GEOTEK CONSORTIUM**

**Office No. 3, Ground Floor,  
Joybelle Apartments  
Mahim, Mumbai - 16.**



**GEOTECHNICAL INVESTIGATION REPORT**  
**PROPOSED DEVELOPMENT**  
**PALGHAR, BHOISAR (E)**  
**FOR M/S. U.S. HOLIDAY REALTY PVT. LTD.**

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**GEOTECHNICAL INVESTIGATION REPORT  
PROPOSED DEVELOPMENT  
PALGHAR, BHOISAR (E)  
FOR M/S. U.S. HOLIDAY REALTY PVT. LTD.**

**1.0 INTRODUCTION**

M/s. U. S. Holiday Realty Pvt. Ltd. plans development of a plot in Bhoisar (E). The work of geotechnical investigation was awarded to M/s. Drill Tech Engineers. The field work and laboratory tests for the geotechnical investigation were completed by M/s. Drill Tech Engineers in January 2008. This report prepared by Geotek Consortium presents results of the geotechnical investigation along with foundation recommendations for the proposed development.

**2.0 EXPLORATION PROGRAM**

**2.1 Exploration Scope**

Eight Boreholes (BH-1 to BH-8) were completed for the project as illustrated on the Borehole Location Plan in the Annexure.



## **2.2 Subsurface Conditions**

Subsurface profile at this site generally consists of residual soils underlain by completely weathered bedrock and then Bedrock. Encountered soil/rock layers are described below;

### **LAYER I: RESIDUAL SOILS**

Residual soils, consisting mostly of yellowish brown clay, were encountered directly from the ground surface in the boreholes. This layer is formed by the complete in-place disintegration of parent bedrock material to texture of soil. Based on Standard Penetration Tests (SPT), consistency of the cohesive soils, was very stiff. The lower boundary of this layer was encountered at depths between 2.0m and 3.0m below ground surface.

### **LAYER III: COMPLETELY WEATHERED BEDROCK (HARD MURRUM)**

Completely weathered bedrock was encountered at depths between 2.0m and 3.0m below ground surface. This layer is formed by the complete in-place disintegration of parent bedrock material, but still partially retains the original rock mass structure and is also locally known as hard murrum. Core Recoveries and Rock Quality Designation (RQD) were nil. SPT tests conducted in this layer encountered refusals. The lower boundary of this layer was encountered at depths between 2.0m and 6.0m below ground surface.





### LAYER III: BASALT BEDROCK

Brownish Gray Basalt bedrock was encountered at depths between 2.0m and 6.0m below ground surface in the boreholes. The bedrock was highly weathered to sound, generally improving with depth. Core Recoveries varied between 20% and 98%, while Rock Quality Designation (RQD) ranged between 0% and 98%. Compressive strength of rock core samples ranged between 400 kg/cm<sup>2</sup> and 775 kg/cm<sup>2</sup>. The boreholes were terminated in this bedrock layer at depths between 7.0m and 15.50m below ground surface.

### 2.3 Ground water Levels

Groundwater accumulation in boreholes was monitored during and after completion of drilling activities. Groundwater was observed in boreholes at a depth of approximately 5.0m below ground surface. Seasonal and annual fluctuations in ground water levels can be expected.



### **3.0 FOUNDATION RECOMMENDATIONS**

Completely weathered bedrock (hard murrum) was encountered at depths typically between 2.0m and 3.0m below ground surface. Spread/raft foundations for the proposed structures, supported on this strata at a depth of 3.0m below ground surface, or on bedrock if encountered earlier, can be designed for a maximum net allowable bearing pressure of 40 t/m<sup>2</sup>.

Minimum footing width should be 1.0m. Maximum settlement of foundations will be less than 10mm. A modulus of subgrade reaction of 4,000 t/m<sup>3</sup> can be utilized for design of foundations.

Excavation sides should be sloped at a maximum slope of 1:2 (horizontal:vertical) or flatter. Dewatering will be required in footing excavations. Excavations should not be kept exposed for prolonged periods of time prior to pouring of PCC. Excavated soils are suitable for use as footing backfill.



### **3.1 Foundation Protection**

Results of chemical analysis on groundwater samples enclosed in the Annexure, indicate that the site falls under Class 1 for sulphate and chloride concentrations (As per IS456 and as per CIRIA Sp. Publication No. 31). A 'moderate' Exposure Condition was assigned to this site. Therefore only following normal precautions are recommended to protect subsurface concrete and reinforcement.

Type of Cement:	OPC or PPC
Minimum Grade of Reinforced Concrete:	M25
Minimum Cement Content for spread footings:	300 kg/m <sup>3</sup>
Maximum Water Cement Ratio:	0.50
Minimum Cover to Reinforcement:	50mm



#### 4.0 FIELD EXPLORATION PROCEDURES

The sub-surface investigation was completed generally as per IS: 1892-1979. The field investigation was carried out using a rotary machine. Casing was used to support sides of borehole until sufficiently stiff strata was encountered. Standard Penetration Tests (i.e. SPT) were carried out in soil in accordance with IS 2131-1981. Using this procedure, a 2" outside diameter split-barrel sampler is driven into the soil by 63.5 kg. weight falling through 75 cm height. After an initial set of 15cm, the number of blows required to drive the sampler an additional 30 cm, is known as the "penetration resistance" or "N value".

When SPT refusal was obtained in hard strata, rock coring was done using diamond bit and double tube core barrel to obtain rock samples. Percent Rock Core Recovery and Rock Quality Designation (%RQD) were determined.  $\% RQD = 100 \times \text{Sum of length of rock pieces in cms, each having lengths greater than 10cms} / \text{Total length of core run.}$

Sincerely,

GEOTEK CONSORTIUM

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Jaydeep Wagh  
B.E., M.S., P.E. (Geotechnical)





## REFERENCES

- 1) Foundation Analysis and Design, J.E. Bowles, McGraw Hill Publication, 5<sup>th</sup> Edition, 1996.
- 2) Canadian Foundation Engineering Manual.
- 3) Soil Mechanics in Engineering Practice, 2<sup>nd</sup> Edition, Terzaghi K. and Peck R. B., John Wiley and Sons, 1967.
- 4) Foundation Design Manual, N. V. Nayak, 5<sup>th</sup> Edition, 1996.
- 5) IS:6403-1981, Code of Practice for Design and Construction of Shallow Foundations on Soils.

CLIENT : **U S HOLIDAY REALITY PVT LTD.**

PROJECT : SOIL INVESTIGATION REPORT AT BOISAR(E)






























BORE HOLE NO. : ,BH NO -02

Date:-28/10/2008 TO 31/10/2008

Rock Started at 6.00M

Water Level at : 4.90M

Final Depth 7.00M G.W.L.

DEPTH (m.)	DIA. OF BORE HOLE	LOG.	STRATA DESCRIPTION	SAMPLE		BLOWS/15cm				SPT N	C R %	RQD %	OTHER TESTS
				DEPTH (m)	TYPE	25	50	75	100				
1.00	NX		BROWNISH SANDY CLAY WITH GRAVELS										
													
				1.50									
				1.75	DS-1	15	10			N			
2.00			BROWNISH GRAY BESALT ROCK		SPT1	17	50			750			
													
													
3.00				3.00	1-3						39%	8%	
													
													
4.00													
													
													
5.00													
													
													
6.00				6.00	8-12						35%	NIL	
													
													
7.00				7.00	13-17						55%	30%	
													
													
8.00													
													
													
9.00													
													
													
10.00													

SPT N = STANDARD PENETRATION TEST VALUE  
CR = COPE RECOVERY

RQD = ROCK QUALITY DESIGNATION  
DS = DISTURBED SOIL SAMPLE

UDS = UNDISTURBED SOIL SAMPLE  
VST = VANE SHEAR TEST

REMARKS : BORE HOLE IS TERMINATED AT DEPTH 7.00M BELOW G.L

SCALE : 1: 50  
JOB NO. :

**DRILLTECH ENGINEERS**

Tel : 2500 4921 / 2500 4922 • Fax : 2500 4923 • Mobile : 98203 05051





### **SAMPLE CALCULATION OF ALLOWABLE BEARING CAPACITY FOR FOUNDATIONS ON COMPLETELY WEATHERED BEDROCK**

	GL +0.0m
Layer I, Residual soils (Clay)	-2.0m to -3.0m
Layer II, Completely weathered Bedrock	-2m to -6.0m
Layer III, Basalt Bedrock	

**(Assuming Completely weathered Bedrock to be a very dense granular soil.)**

Net Ultimate Bearing Capacity =  $q_u = cN_c + q(N_q - 1) + 0.5 B \gamma N_\gamma s_\gamma$  (Refn. 5, Table 4-1)

Where,

- $q$  = Overburden Pressure (i.e. submerged unit weight x depth of foundation)
- $c$  = Cohesion
- $B$  = Minimum Width of foundation = 1m
- $\gamma'$  = submerged unit weight of soil = 0.80
- $N_c, N_q, N_\gamma$  = Terzaghi's Bearing capacity factors
- $S_c, s_q, s_\gamma$  = Shape factors = 1, 1, and 0.8, respectively
- $D$  = Minimum Depth of Footing = 2.0m

Minimum SPT N value obtained in the completely weathered bedrock = 50

Corresponding friction angle =  $40^\circ$  (Reference No. 5)

Corresponding  $N_c=75$ ,  $N_q=64$ ,  $N_\gamma=109$  (Reference 5, IS:6403-1981);

Substituting these values in the above equation;

$$q_{\text{ultimate}} = q_u = [0 \times 75 \times 1] + [2 \times 0.8 \times (64 - 1) \times 1] + [0.5 \times 1 \times 0.8 \times 109 \times 0.6] = 0 + 100 + 25 = 125 \text{ t/m}^2$$

$$q_{\text{safe}} = q_u / \text{F.S.} = 125 / 3 = 41 \text{ t/m}^2$$





**CALCULATION OF SETTLEMENTS OF FOUNDATIONS (3M X 3M) EXERTING PRESSURE OF 40 T/M2:**

From Reference No. 1:

$$\text{Settlement} = S = q_0 B' \frac{1 - \mu^2}{E_s} m I_s I_f$$

Where,

$q_0$  = Footing Pressure = 40 t/m<sup>2</sup>

$B'$  =  $B/2$  (Where  $B$  is the width of pressure distribution)

$\mu$  = Poisson's ratio = 0.3

$E$  = Modulus of Elasticity

$I_s$  = Influence Factor (Obtained from Table 5-2, Reference No. 1)

$I_f$  = Depth Factor (Obtained from Figure 5-7, Reference No. 1)

$m$  = 4 for center of footing

Very conservatively assuming weathered bedrock within the full influence zone of footings:

$E$  value for over-consolidated sand =  $105(N) + 4000$  (Reference No. 1)

Therefore, for a SPT  $N$  value of 50,  $E = 9,250$  t/m<sup>2</sup>

$L' = 3/2 = 1.50$ ,  $B' = 3/2 = 1.5$ ,  $H = 6\text{m}$ , and  $D = 2.0\text{m}$

Therefore,  $M = L/B = 1$ ; and  $N = H/B' = 4$  and  $D/B = 0.67$

Corresponding,  $I_s = 0.43$ , Conservative  $I_f = 1.0$  (From Table 5-2, Reference 1)

$$\text{Settlement of Layer} = S_1 = 40 \times 1.5 \times \frac{1 - 0.3^2}{9250} \times 4 \times 0.43 \times 1.0 = 0.011\text{m} = 11\text{mm}$$

From IS8009:

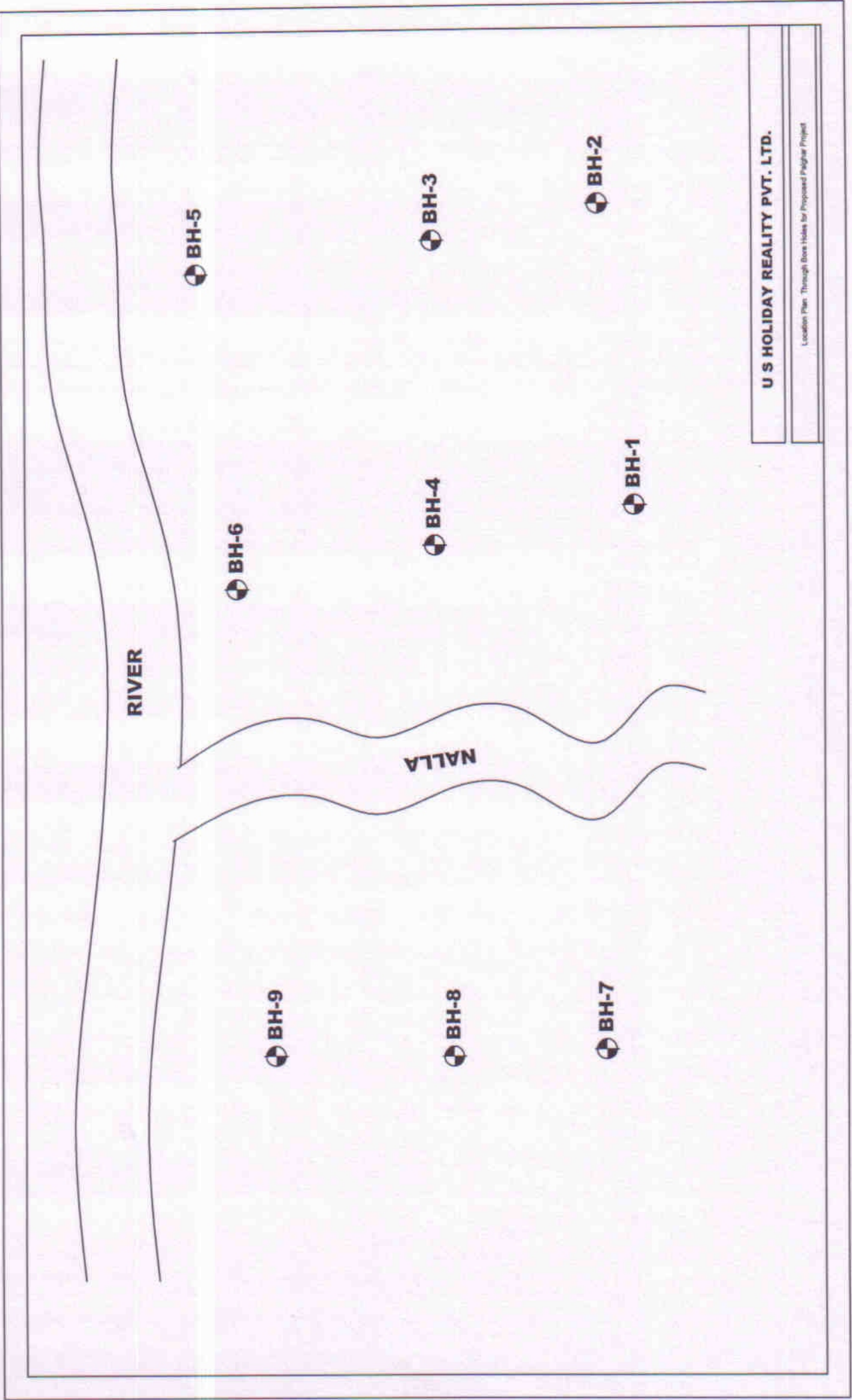
Due to Footing Rigidity Factor, Settlement =  $0.8 \times 11\text{mm} = 9\text{mm}$

**Therefore, Total Settlement = 9mm**



# ANNEXURES

# **BOREHOLE LOCATION PLAN**



U S HOLIDAY REALITY PVT. LTD.
Location Plan Through Bore Holes for Proposed Padghe Project

# **BOREHOLE LOGS**



# **SUBSURFACE PROFILE**

CLIENT : <b>U S HOLIDAY REALITY PVT LTD.</b>													
PROJECT : SOIL INVESTIGATION REPORT AT :PALGHAR BOISAR(E)													
BORE HOLE NO. : BH NO -01							Date:- 22/10/2008 TO 24/10/2008						
Rock Started at 1.70M				Water Level at :5.00M				Final Depth 6.80M G.W.L.					
DEPTH (m.)	DIA. OF BORE HOLE	LOG.	STRATA DESCRIPTION	SAMPLE		BLOWS/15cm				SPT N	C R %	RQD %	OTHER TESTS
				DEPTH (m)	TYPE	25	50	75	100				
1.00	NX		BROWNISH SANDY CLAY WITH GRAVELS	1.50	DS-1								
				1.70	SPT1	15	10			N >50			
2.00													
3.00			BROWNISH GRAY WEATHERED BESALT ROCK	3.00	1-3						23%	NIL	
4.00				4.50	4-10						42%	NIL	
5.00			BROWNISH GRAY BESALT ROCK	6.00	11-13						49%	13%	
6.00				6.80	14-16						69%	55%	
7.00													
8.00													
9.00													
10.00													

SPT N =STANDARD PENETRATION TEST VALUE  
CR = CORE RECOVERY

RQD = ROCK QUALITY DESIGNATION  
DS = DISTURBED SOIL SAMPLE

UDS = UNDISTURBED SOIL SAMPLE  
VST = VANE SHEAR TEST

SPT N = STANDARD PENETRATION TEST VALUE  
CR = CORE RECOVERY

RQD = ROCK QUALITY DESIGNATION  
DS = DISTURBED SOIL SAMPLE

UDS = UNDISTURBED SOIL SAMPLE  
VST = VANE SHEAR TEST

REMARKS : BORE HOLE IS TERMINATED AT DEPTH 6.80M BELOW G.L.  
**DRILLTECH ENGINEERS**

SCALE : 1: 50  
JOB NO. :



CLIENT : **U S HOLIDAY REALITY PVT LTD.**

PROJECT : SOIL INVESTIGATION REPORT AT BOISAR(E)


BORE HOLE NO. : ,BH NO -03

Date:-28/10/2008 TO 31/10/2008

Rock Started at 6.00M

Water Level at : 4.90M

Final Depth 7.00M G.W.L.

DEPTH (m.)	DIA. OF BORE HOLE	LOG.	STRATA DESCRIPTION	SAMPLE		BLOWS/15cm					SPT N	C R %	RQD %	OTHER TESTS
				DEPTH (m)	TYPE	25	50	75	100					
1.00	NX		BROWNISH SANDY CLAY WITH GRAVELS	1.50										
					DS-1	15	15	05		N				
				1.85	SPT1	10	13	50		>50		71%		
2.00														
3.00														
3.00				3.00	DS-1									
				3.20	SPT2	15	05			N		75%		
						18	50			>50				
4.00			BROWNISH STIFF SILTY CLAY WITH MURRUN											
4.50				4.50	DS-1									
				4.75	SPT3	15	10			N		80%		
						13	52			>50				
5.00														
6.00				6.00										
			BROWNISH GRAY WEATHERED BESALT ROCK											
7.00				7.00	1-2						25%	NIL		
			BROWNISH GRAY BESALT ROCK											
8.00				8.00	3-6						87%	85%		
9.00				9.00	7-8					64%	64%			
10.00				10.00	9-10					86%	83%			
				11.00	11-15					72%	59%			

SPT N = STANDARD PENETRATION TEST VALUE  
CR = CORE RECOVERY

RQD = ROCK QUALITY DESIGNATION  
DS = DISTURBED SOIL SAMPLE

UDS = UNDISTURBED SOIL SAMPLE  
VST = VANE SHEAR TEST

REMARKS : BORE HOLE IS TERMINATED AT DEPTH 11.00M BELOW G.L

SCALE : 1: 50  
JOB NO. :

**DRILLTECH ENGINEERS**

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CLIENT : **U S HOLIDAY REALITY PVT LTD.**

PROJECT : SOIL INVESTIGATION REPORT ATBOISAR(E)


BORE HOLE NO. : BH-03

Date: 28/10/2008 TO 31/10/2008

Rock Started at 6.00M

Water Level at : 4.90

Final Depth 7.00 G.W.L.

DEPTH (m.)	DIA. OF BORE HOLE	LOG.	STRATA DESCRIPTION	SAMPLE		BLOWS/15cm				SPT N	C R %	RQD %	OTHER TESTS
				DEPTH (m)	TYPE	25	50	75	100				
11.00	NX	▽▽▽▽	BROWNISH GRAY BESALT ROCK	11.00	11-15						72%	59%	
12.00													
13.00													
14.00													
15.00													
16.00													
17.00													
18.00													
19.00													
20.00													

SPT N = STANDARD PENETRATION TEST VALUE  
CR = CORE RECOVERY

RQD = ROCK QUALITY DESIGNATION  
DS = DISTURBED SOIL SAMPLE

UDS = UNDISTURBED SOIL SAMPLE  
VST = VANE SHEAR TEST

REMARKS : BORE HOLE IS TERMINATED AT DEPTH 11.00M BELOW G.L.

SCALE : 1: 50  
JOB NO. :



CLIENT : **U S HOLIDAY REALITY PVT LTD.**

PROJECT : SOIL INVESTIGATION REPORT AT BOISAR(E)



BORE HOLE NO. : ,BH NO -04

Date:-2/11/2008 TO 4/11/2008

Rock Started at 3.10M

Water Level at :5.00M

Final Depth 8.00M G.W.L.

DEPTH (m.)	DIA. OF BORE HOLE	LOG.	STRATA DESCRIPTION	SAMPLE		BLOWS/15cm				SPT N	C R %	RQD %	OTHER TESTS
				DEPTH (m)	TYPE	25	50	75	100				
1.00	NX		BROWNISH SANDY CLAY WITH GRAVELS	1.50									
					DS-1	15	15	15	05	N			
2.00				2.00	SPT1	7	8	50	52	18			
			BROWNISH GRAY WEATHERED ROCK	3.00	DS-1								
3.00				3.10	SPT2	10	52			N >50	80%		N
4.00				4.50	1-2						NIL	NIL	
5.00			BROWNISH GRAY WEATHERED BESALT ROCK	6.00	3-6						70%	67%	
6.00				7.00	7-9						67%	49%	
7.00				8.00	10-11						88%	88%	
8.00													
9.00													
10.00													

SPT N = STANDARD PENETRATION TEST VALUE

CR = CORE RECOVERY

RQD = ROCK QUALITY DESIGNATION

DS = DISTURBED SOIL SAMPLE

UDS = UNDISTURBED SOIL SAMPLE

VST = VANE SHEAR TEST

REMARKS : BORE HOLE IS TERMINATED AT DEPTH 8.00M BELOW G.L

SCALE : 1: 50

JOB NO. :

**DRILLTECH ENGINEERS**

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**DRILLTECH ENGINEERS**  
**DE**



CLIENT : **U S HOLIDAY REALTY PVT LTD.**

PROJECT : SOIL INVESTIGATION REPORT AT :PALGHAR BOISAR(E)

BORE HOLE NO. : BH NO -05

Date:- 6/11/2008 TO 8/11/2008

Rock Started at 3.00M

Water Level at :5.00M

Final Depth 9.00M G.W.L.

DEPTH (m.)	DIA. OF BORE HOLE	LOG.	STRATA DESCRIPTION	SAMPLE		BLOWS/15cm					SPT N	C R %	RQD %	OTHER TESTS
				DEPTH (m)	TYPE	25	50	75	100					
1.00	NX		BROWNISH SANDY CLAY WITH GRAVELS	1.50										
2.00					DS-1	15	15	15	15	N				
				2.10	SPT1	8	7	13	12	20				
3.00				3.00										
4.00					BROWNISH GRAY BESALT ROCK	4.50							17%	NIL
5.00														
6.00	6.00	1-3								23%	NIL			
7.00	7.00	4-5								78%	36%			
8.00	8.00	8-9								56%	36%			
9.00	9.00	9-11								53%	25%			
10.00														

SPT N =STANDARD PENETRATION TEST VALUE

CR = CORE RECOVERY

RQD = ROCK QUALITY DESIGNATION

DS = DISTURBED SOIL SAMPLE

UDS = UNDISTURBED SOIL SAMPLE

VST = VANE SHEAR TEST

REMARKS : BORE HOLE IS TERMINATED AT DEPTH 9.00M BELOW G.L

SCALE : 1: 50

JOB NO. :

**DRILLTECH ENGINEERS**





CLIENT : **U S HOLIDAY REALTY PVT LTD**

PROJECT : SOIL INVESTIGATION REPORT AT BOISAR(E)








BORE HOLE NO. : ,BH NO -06

Date:- 9/11/2008 TO 11/11/2008

Rock Started at 2.00M

Water Level at : 5.20M

Final Depth 7.00M G.W.L.

DEPTH (m.)	DIA. OF BORE HOLE	LOG.	STRATA DESCRIPTION	SAMPLE		BLOWS/15cm				SPT N	C R %	RQD %	OTHER TESTS
				DEPTH (m)	TYPE	25	50	75	100				
1.00	NX		BROWNISH SANDY CLAY WITH GRAVELS	2.00									
2.00													
3.00			BROWNISH GRAY FACHTRED BASALT ROCK	3.50	1-2						20%	NIL	
4.00													
5.00			BROWNISH GRAY BESALT ROCK	5.00	3-4						28%	NIL	
6.00				6.00	4-7						38%	NIL	
7.00				7.00	8-15						63%	48%	
8.00													
9.00													
10.00													

SPT N = STANDARD PENETRATION TEST VALUE  
CR = CORE RECOVERY

RQD = ROCK QUALITY DESIGNATION  
DS = DISTURBED SOIL SAMPLE

UDS = UNDISTURBED SOIL SAMPLE  
VST = VANE SHEAR TEST

REMARKS: BORE HOLE IS TERMINATED AT DEPTH 7.00M BELOW G.L

SCALE : 1: 50  
JOB NO. :

**DRILLTECH ENGINEERS**





CLIENT : **U S HOLIDAY REALTY PVT LTD.**

PROJECT : SOIL INVESTIGATION REPORT AT BOISAR(E)




BORE HOLE NO. : ,BH NO -07

Date:-12/11/2008 TO 14/11/2008

Rock Started at 6.00M

Water Level at : 5.50M

Final Depth 11.00MG.W.L.

DEPTH (m.)	DIA. OF BORE HOLE	LOG.	STRATA DESCRIPTION	SAMPLE					BLOWS/15cm	SPT N	C R %	RQD %	OTHER TESTS
				DEPTH (m)	TYPE	25	50	75	100				
1.00	NX		BROWNISH SANDY CLAY WITH GRAVELS	1.50									
					DS-1	15	15	15	15	N			
2.00				2.10	SPT1	7	8	11	13	19			
3.00			BROWNISH CLAY WITH MURRUM & BOULDERS	3.00							NIL	NIL	
4.00				4.50									
5.00													
6.00				6.00									
7.00			BROWNISH GRAY WEATHERED BESALT ROCK	7.50							15%	NIL	
8.00													
9.00				9.00	1-4						33%	NIL	
10.00				10.00	5-10						88%	80%	
				11.00	11-16						67%	54%	

SPT N = STANDARD PENETRATION TEST VALUE  
CR = CORE RECOVERYRQD = ROCK QUALITY DESIGNATION  
DS = DISTURBED SOIL SAMPLEUDS = UNDISTURBED SOIL SAMPLE  
VST = VANE SHEAR TEST


REMARKS : BORE HOLE IS TERMINATED AT DEPTH 11.00M BELOW G.L

SCALE : 1: 50  
JOB NO. :**DRILLTECH ENGINEERS**

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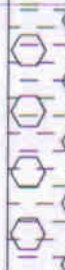




CLIENT : <b>U S HOLIDAY REALTY PVT LTD</b>													
PROJECT : SOIL INVESTIGATION REPORT ATBOISAR(E)													
BORE HOLE NO. : BH-07						Date: 12/11/2007 TO 14/11/2008							
Rock Started at 6.00M			Water Level at : 5.50				Final Depth 11.00 G.W.L.						
DEPTH (m.)	DIA. OF BORE HOLE	LOG.	STRATA DESCRIPTION	SAMPLE		BLOWS/15cm				SPT N	C R %	RQD %	OTHER TESTS
				DEPTH (m)	TYPE	25	50	75	100				
11.00	NX	▽ ▽ ▽ ▽ ▽ ▽ ▽ ▽ ▽ ▽	BROWNISH GRAY BESALT ROCK	11.00	11-16						67%	54%	
12.00													
13.00													
14.00													
15.00													
16.00													
17.00													
18.00													
19.00													
20.00													
<div> <div>SPT N = STANDARD PENETRATION TEST VALUE</div> <div>CR = CORE RECOVERY</div> </div> <div> <div>RQD = ROCK QUALITY DESIGNATION</div> <div>DS = DISTURBED SOIL SAMPLE</div> </div> <div> <div>UDS = UNDISTURBED SOIL SAMPLE</div> <div>VST = VANE SHEAR TEST</div> </div>													
REMARKS : BORE HOLE IS TERMINATED AT DEPTH 11.00M BELOW G.L											SCALE : 1: 50		
DRILLTECH ENGINEERS											JOB NO. :		
Tel.: 2500 4921 / 2500 4922 • Fax : 2500 4923 • Mobile : 98203 05051													





CLIENT : <b>U S HOLIDAY REALTY PVT LTD.</b>													
PROJECT : SOIL INVESTIGATION REPORT AT BOISAR(E)													
BORE HOLE NO. : ,BH NO -08							Date:-16/11/2008 TO 18/11/2008						
Rock Started at 6.00M				Water Level at :5.50M				Final Depth 15.50MG.W.L.					
DEPTH (m.)	DIA. OF BORE HOLE	LOG.	STRATA DESCRIPTION	SAMPLE		BLOWS/15cm				SPT N	C R %	RQD %	OTHER TESTS
				DEPTH (m)	TYPE	25	50	75	100				
1.00	NX		BROWNISH SANDY CLAY WITH GRAVELS	1.50									
					DS-1	15	15	15	05	N			
2.00				2.00	SPT1	8	10	13	52	23			
			BROWNISH GRAY WITH MURRUM										
3.00				3.00	-						NIL	NIL	
			BROWNISHMURRUM WITH BOULDERS										
4.00				4.50	-						NIL	NIL	
5.00													
6.00			6.00	-							NIL	NIL	
	BROWNISH GRAY HIGHLY WEATHERED ROCK												
7.00		7.50	1-2							8%	NIL		
	HIGHLY TOMODERATLY WEATHERED ROCK												
8.00			small pcs							13%	NIL		
9.00		9.00											
			small pcs										
10.00	10.50									19%	NIL		
	12.00	3-4								22%	NIL		
SPT N=STANDARD PENETRATION TEST VALUE CR = CORE RECOVERY RQD = ROCK QUALITY DESIGNATION DS = DISTURBED SOIL SAMPLE UDS = UNDISTURBED SOIL SAMPLE VST = VANE SHEAR TEST													
REMARKS : BORE HOLE IS TERMINATED AT DEPTH 15.50M BELOW G.L												SCALE : 1: 50 JOB NO. :	



CLIENT : <b>U S HOLIDAY REALTY PVT LTD.</b>													
PROJECT : SOIL INVESTIGATION REPORT ATBOISAR(E)													
BORE HOLE NO. : BH-08						Date: 19/11/2008 TO 18/11/2008							
Rock Started at 6.00M				Water Level at : 5.50				Final Depth 15.50M.W.L.					
DEPTH (m.)	DIA. OF BORE HOLE	LOG.	STRATA DESCRIPTION	SAMPLE		BLOWS/15cm				SPT N	C R %	RQD %	OTHER TESTS
				DEPTH (m)	TYPE	25	50	75	100				
11.00	NX	▽▽▽▽ ▽▽▽▽ ▽▽▽▽ ▽▽▽▽ ▽▽▽▽	HIGHLY TOMODERATETLY WEATHERED ROCK	12.00	3-4						22%	NIL	
13.00				BROWNISH GRAY BASALT ROCK	13.50	5-12						25%	NIL
14.00			15.00		13-17						43%	27%	
15.00						15.50	18						98%
16.00													
17.00													
18.00													
19.00													
20.00													

SPT N = STANDARD PENETRATION TEST VALUE  
CR = CORE RECOVERY




RQD = ROCK QUALITY DESIGNATION  
DS = DISTURBED SOIL SAMPLE

UDS = UNDISTURBED SOIL SAMPLE  
VST = VANE SHEAR TEST

REMARKS : BORE HOLE IS TERMINATED AT DEPTH 15.50M BELOW G.L

SCALE : 1: 50  
JOB NO. :



CLIENT : <b>U S HOLIDAY REALTY PVT LTD.</b>														
PROJECT : SOIL INVESTIGATION REPORT AT BOISAR(E)														
BORE HOLE NO. : ,BH NO -09						Date:20/11/2008 TO 22/11/2008								
Rock Started at 6.00M				Water Level at : 5.00M				Final Depth 12.50MG.W.L.						
DEPTH (m.)	DIA. OF BORE HOLE	LOG.	STRATA DESCRIPTION	SAMPLE		BLOWS/15cm				SPT N	C R %	RQD %	OTHER TESTS	
				DEPTH (m)	TYPE	25	50	75	100					
1.00	NX		BROWNISH SANDY CLAY WITH GRAVELS	1.50	DS-1									
1.70				SPT1	15	05			N					
					13	52		>52						
2.00														
3.00						3.00						NIL	NIL	
4.00					BROWNISH MURRUM WITH BOULDERS	4.50						NIL	NIL	
5.00														
6.00						6.00						NIL	NIL	
7.00					BROWNISH GRAY HIGHLY WEATHERED ROCK	7.50	1-2					12%	NIL	
8.00														
9.00					9.00	3-5					17%	NIL		
10.00			BROWNISH GRAY BESALT ROCK	10.50	6-9					23%	NIL			
					12.00	10-12					39%	10%		

SPT N = STANDARD PENETRATION TEST VALUE  
CR = CORE RECOVERY

RQD = ROCK QUALITY DESIGNATION  
DS = DISTURBED SOIL SAMPLE

UDS = UNDISTURBED SOIL SAMPLE  
VST = VANE SHEAR TEST

REMARKS : BORE HOLE IS TERMINATED AT DEPTH 12.50M BELOW G.L.

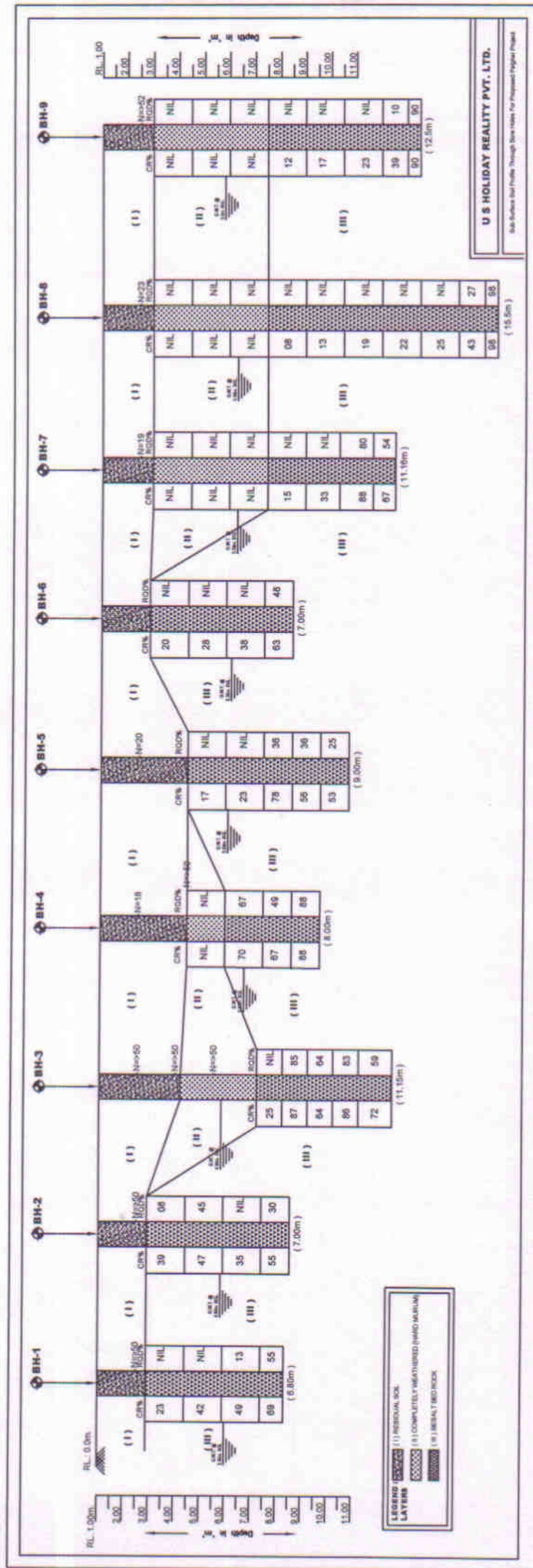
SCALE : 1: 50  
JOB NO.:





# **SUBSURFACE PROFILE**







# **LABORATORY TEST RESULTS**

## GRAIN SIZE ANALYSIS

Laboratory testing for U S Holidays, Boisar (E)

**Project:**

**Job No.** GSL-413

**Date:** 2/1/2009

Symbol	Particle size mm	CLAY	SILT	F	SAND	M	C	F	GRAVEL	C
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Bore Hole No.	BH-3	BH-8								
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Depth (m)	3.00-3.20	0.00-1.50								
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Sample Type	SPT	DS-1								
-------------	-----	------	--	--	--	--	--	--	--	--

Classification	CH	CH								
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Gravel %	0	4								
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Sand %	5	10								
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Silt %	95	86								
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Clay %	<---->	<---->								
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D60 mm	---	---								
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D30 mm	---	---								
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D10 mm	---	---								
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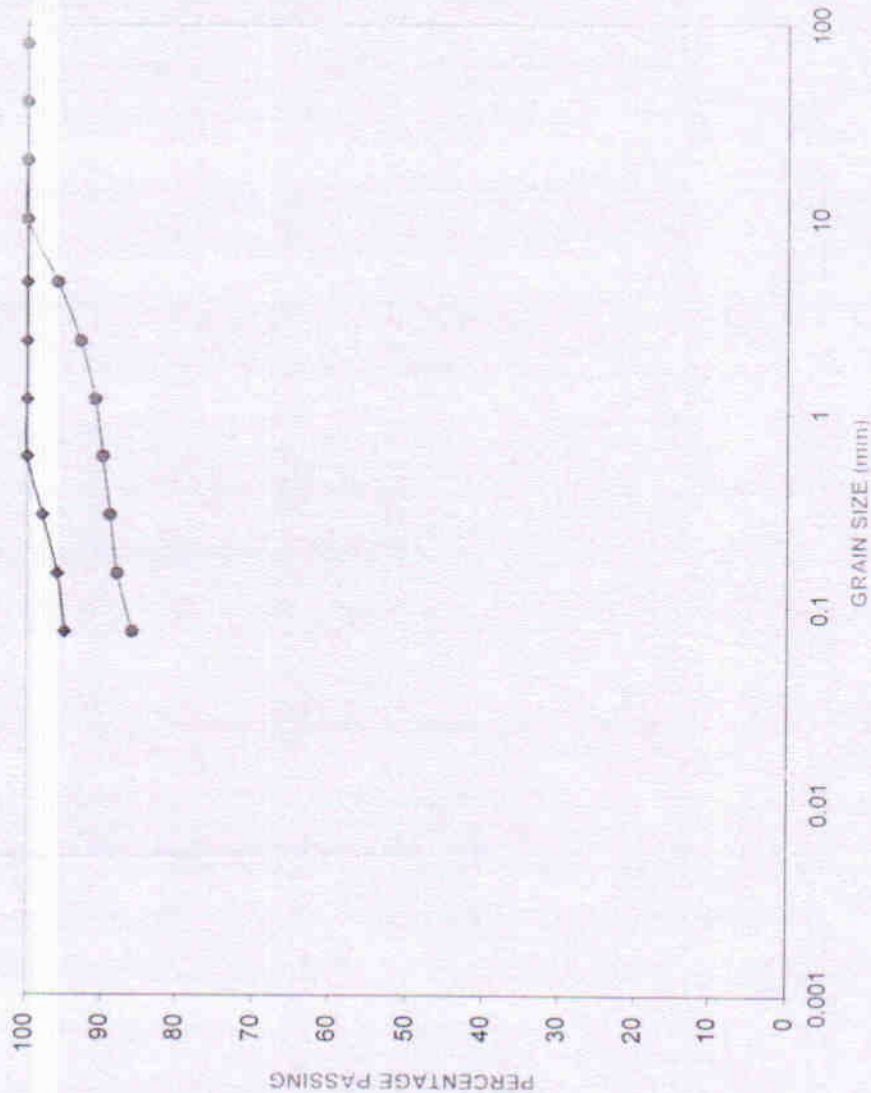
Cu	---	---								
----	-----	-----	--	--	--	--	--	--	--	--

Cc	---	---								
----	-----	-----	--	--	--	--	--	--	--	--

Liquid Limit %	78	58								
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Plastic Limit %	24	25								
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Plasticity Index	54	33								
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## Laboratory testing for U S Holidays, Boisar (E)

Job No.:  
GSL-413

Date: 2/1/2009

PCP : Preconsolidation Pressure  
 <----> : Combined percentage of silt & clay  
 NP : Non Plastic

UU: Unconsolidated Undrained triaxial test  
CU: Consolidated Undrained triaxial test  
CD: Consolidated Drained triaxial test  
UC: Unconfined Compressive strength test  
DST: Direct Shear Test

UDS: Undisturbed sample  
NMC: Natural moisture content  
C: Cohesion  
 $\phi^0$ : Angle of internal friction

DRILLTECH ENGINEERS

GEO SCIENCE LABORATORY



## ROCK TEST RESULTS

Name of the Client : Laboratory testing for U S Holidays											Date : 2/1/2009		
Location : Boisar (E)											Job No.: GSL-413		
Sr. No.	Bore Hole no.	Core No.	Depth (m)	Diameter (cm)	Height (cm)	H/D	Crushing Load (kg)	Point Load Index (kg/cm <sup>2</sup> )	UCS (kg/cm <sup>2</sup> )	Water absorption (%)	Porosity (%)	Dry density (gm/cc)	Remarks
1	BH-1	3	3.00-4.50	5.21	---	---	50	1.84	---	1.86	4.20	2.26	---
2	BH-1	14	6.80	5.64	---	---	950	29.87	---	0.21	0.49	2.32	---
3	BH-2	3	3.00	5.47	8.11	1.48	12000	---	473.74	0.68	1.57	2.31	---
4	BH-3	2	6.00-7.00	5.41	7.61	1.41	19500	---	775.35	0.26	0.61	2.35	---
5	BH-4	3	4.50-6.00	5.41	10.79	1.99	14500	---	630.15	0.31	0.72	2.31	---
6	BH-5	8	7.00-8.00	5.40	10.78	2.00	16500	---	719.87	0.27	0.63	2.33	---
7	BH-6	15	6.00-7.00	5.41	7.51	1.39	13000	---	514.92	1.02	2.35	2.30	---
8	BH-7	14	10.00-11.00	5.97	9.11	1.53	20500	---	684.82	0.68	1.57	2.31	---
9	BH-8	10	13.50-15.00	5.47	10.86	1.99	9500	---	403.44	1.72	3.94	2.29	---
10	BH-9	6	10.50-12.00	5.41	9.12	1.69	14500	---	605.48	0.81	1.86	2.30	---

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DRILLTECH ENGINEERS

Date: 2/1/2009

Project: Laboratory testing for U S Holidays

Site: Boisar (E)

Job No.: GSL -413

CHEMICAL ANALYSIS OF SOIL SAMPLE

	BH-3	BH-8
	3.00-3.20	0.00-1.50
pH value	7.64	7.68
Sulphate as So3 (%)	0.039	0.052
Chloride as cl (%)	0.083	0.128

CHEMICAL ANALYSIS OF WATER SAMPLE

	BH-1	BH-7
pH value	7.63	7.62
Sulphate as So3 (ppm)	58	52
Chloride as cl (ppm)	127	112